

February 11, 2019

Please find the following addendum to the below mentioned BID.

Addendum No.: 2

Bid#: 300-00-18-66-2

Project Name: Bootlegger Road Improvements

Bid Due Date: Thursday, February 14, 2019

GENERAL INFORMATION:

- Section 03 Summary of Work Revised IV. Other Requirements (as applicable) to reflect the Louisiana Department of Transportation and Development (2016 Edition of Louisiana Standard Specifications for Roads and Bridges). Remove "Section 03-Summary of Work" in its entirety and replace it with "Section 03-Summary of Work Revised". (Attached)
- 2. Add Geotechnical Engineering Services Report to Section 03–Summary of Work.(Attached)
 - Summary of Drainage Structures table that correlates to these subsurface drainage improvements.
- 3. Add Summary of Coquille Drainage Structures Table to Section 03-Summary of Work. (Attached)
- 4. Please remove the "Section 04 Louisiana Uniform Public Work Bid Form" in its entirety and replace with "Section 04 Louisiana Uniform Public Work Bid Form Revised". (Attached)
- Please remove "Louisiana Uniform Public Work Bid Form Unit Price Form" and replace with "Louisiana Uniform Public Work Bid Form Unit Price Form Revised". (Attached) *<u>Original</u> Alternate No. 1 has been removed from this project altogether and replaced with the <u>new</u> Alternate #1 as described below.
 - Added bid items:
 - i. Storm Drain Pipe (15" RPVCP) (701-03-01002)
 - ii. Storm Drain Pipe Arch (42" Equiv. RCPA) (701-04-01100)
 - iii. Storm Drain Pipe Arch (48" Equiv. RCPA) (701-04-01120)
 - iv. Trench Excavation Safety Protection (Depth >5 feet) (701-17-00100)
 - v. Vibration Monitoring (804-18-00100)
 - Added <u>new</u> Alternate #1 Bid Items and Quantities:

PROCUREMENT DEPARTMENT P.O. BOX 628 | COVINGTON, LOUISIANA | 70434 | PROCUREMENT@STPGOV.ORG | 985-898-2520 WWW.STPGOV.ORG

- Description: Alternate #1 provides net quantities that would change the typical section of the Mill and Overlay areas by milling 3.5 inches and overlaying 3.5 inches and removing the polymer based asphalt reinforcement interlayer. NOTE: The polymer based asphalt reinforcement interlayer shall remain for the area of reconstruction as shown in the construction drawings on each side of the bridge replacement.
- Removed bid item for Imprinted Cast-In-Place Concrete System (S-001)
- Removed bid item for Concrete Drive (Colored)(6" Thick) (706-02-10000)
- Revised quantities for bid items:
 - i. Temporary Sediment Check Dams (204-05-00100)
 - ii. Class II Base Course (302-01-00100)
 - iii. Lime (304-01-00100)
 - iv. Lime Treatment (Type D)(12" Thick) (304-04-00400)
 - v. Subgrade Layer (12") (305-01-04000
- Section 10 Technical Specifications Subsection 01150 Measurement and Payment REMOVE 4.01 IMPRINTED CAST-IN-PLACE CONCRETE SYSTEM (S-001) ON PAGE 01150-1 OF SPECIFICATIONS.
- 7. Section 10 Technical Specifications Subsection 03000 Special Items **REMOVE S-001** IMPRINTED CAST-IN-PLACE CONCRETE SYSTEM FROM SPECIFICATIONS.
- 8. Add Plan Sheet 3a- Coquille Drainage Plan. (Attached)
 - Summary of Drainage Structures table that correlates to the Coquille subsurface drainage improvements is located in Section 03 see note 3 of this addendum.
- 9. Remove Sheet 25- Concrete Joint Layout Terra Bella Blvd in its entirety from Construction Drawings.
 - No work shall be performed on Terra Bella Blvd. Mill and Overlay limits shall tie into existing concrete gutter bottom that runs parallel to Bootlegger Road. The entrance is constructed of pavers and shall not be disturbed.
- 10. Geogrid is required between the subbase and base course for the path and sidewalk. This is found in package on plan sheet 2b-2g.
 - Quantities in <u>the Unit Price Bid Form reflect this quantity in SY.</u>
- 11. Glass Interlayer Reinforcement Grid has been <u>denied</u> as an approved equal for the polymer. Glass interlayer is not the same product and does not meet specifications.
- 12. Liquidated damages remains at \$500/day.

QUESTION AND ANSWER:

Question 1:	How many test piles vs production piles; plan and order lengths?
Answer1:	The project calls for dynamic monitoring assistance on two piles, one in the abutment and one on a middle bent for confirmation of pile capacity. Unit Price Sheet- Revised displays Item # 804-14-00100 Dynamic Monitoring Assistance for two piles.
Question 2:	Is there and time restrictions? Night work? Is night work mandatory?
Answer2:	The Parish instructs that there will be no pile driving during night hours. However, operations such as milling, overlay, etc. may be undertaken during after hours, per prior written approval by the Parish. No, night work isn't mandatory.
Question 3:	Extend time to 11 months instead of 9 months?
Answer 3:	270 Calendar Days will remain the time period of this construction contract.
Question 4:	Can mill and overlay be completed prior to bridge replacement construction?
Answer 4:	The sequence of construction is part of the Contractor's means and methods.
Question 5:	The plans do not address the lime stabilization locations. The subgrade layer is shown in the typical section on plan sheet 2e. The lime quantity seems to match the subgrade quantity. Is the lime intended for use in the subgrade layer? Will the subgrade layer require cement, or just lime?
Answer 5:	Yes, the lime is intended for the subgrade layer. The subgrade layer will only require lime.
Question 6:	The Class II base quantity seems to be in agreement with the lime and subgrade layer quantity for the reconstruct area, but there is Class II base shown beneath the shared use path. Where will this base be paid for?
Answer 6:	Quantities for Bid Items – Class II Base Course, Lime, Lime Treatment (Type D) (12" Thick), and Subgrade Layer (12") are on the Unit Price Bid Form Revised.
Question 7:	What it the fee schedule for Cleco ?

Answer 7: CLECO advises that the 34,500 V line along Bootlegger Road is a backup source to the LA 21 commercial corridor line. CLECO can de-energize and re-energize the lines along Bootlegger daily, however a Contractor asked for a fee schedule. The provided fee schedule is as follows:

The cost to the Contractor for Cleco to install and operate the necessary switches to deenergize/ground/re-energize the circuits in the area of the bridge replacement along bootlegger road is \$11,234. This cost is based on the installation of the switches and 2 daily trips to the site for a maximum of five days, if more than 5 days of de-energizing and re-energizing are required, a charge of \$705/day will be assessed against the contractor for each additional day beyond 5 days. The successful Contractor will be required to set up a Cleco account to allow for billing of this cost. This cost does not include any conductor or pole relocations, if necessary. This cost shall be paid under Bid Item # 740-01-00100 Construction Layout – LUMP.

Question 8: What is the project estimate?

Answer 8: Project Estimate is \$2,910,292.00

ATTACHMENTS:

- 1. "Section 03 Summary of Work revised.pdf"
- 2. "Section 03 Geotechnical Engineering Services Report.pdf"
- 3. "Section 03 Summary of Drainage Structures Table.pdf"
- 4. "Section 04 Louisiana Uniform Public Works Form Revised.pdf"
- 5. "Louisiana Uniform Public Work Bid Form Unit Price Form Revised.pdf"
- 6. "Plan Sheet 3a-Coquille Drainage Plan.pdf"

End of Addendum #2

Section 03 Revised Summary of Work

I. Work to Include:

The scope of work for construction includes but is not limited to the following:

- 1) Removal of the timber bridge and replacement with a concrete 3-span bridge over Timber Branch;
- 2) Mill and overlay of approximately 3.2 miles along Bootlegger Road beginning neat LA 1077 and ending near LA 21; and
- 3) Installation of an asphalt shared-use path from Ochsner/Francis Road Roundabout to Resource Bank driveway near LA 21.

Contractor must provide all labor, equipment, tools, and materials necessary for milling, overlaying, asphalt concrete pavement, guardrail, shared-use path, and bridge replacement, as specified.

II. Location of Work:

Bootlegger Rd (R01C018) and Bootlegger Rd Bridge (B01C018) Covington, LA Section 37 Township 6 Range 10E and Section 45 Township 7 Range 10E.

III. <u>Documents:</u> Bid Documents dated December 03, 2018, and entitled:

Bootlegger Road Improvements Bid #300-00-18-66-2

IV. <u>OTHER REQUIREMENTS</u> (as applicable)

When not otherwise specified herein, all work and materials shall conform to the requirements of the Louisiana Department of Transportation and Development hereafter called LDOTD (2016 Edition of Louisiana Standard Specifications for Roads and Bridges) (LSSRB). All work included in these pay items shall be constructed in strict conformance with the appropriate Sections, Sub-sections, and references made within the LSSRB including any current revisions to which as published by the LA Dept. of Transportation and Development and/or as modified, supplemented, or added to which herein. Specifications including materials, equipment, and prosecution of the work, payment, procedures, testing, sampling, quality assurance, and quality control items shall conform to the LSSRB and the latest revised Supplemental Specifications. Payment and measurement will be based on the item and unit measure specified in the LSSRB, or as amended otherwise within this document.

Section 03



Geotechnical Engineering Services Report

Bootlegger Road Mill and Overlay and Bridge Replacement Project Bootlegger Road St. Tammany Parish, Louisiana APS File No: 1702-G009

> Presented to: Stanley Consultants, Inc. 721 Government Street Baton Rouge LA 70802

Prepared by: APS Engineering and Testing, LLC 8000 Innovation Park Dr. Building 3100, Room 250 Baton Rouge, LA 70820

May 16, 2017 Sexc



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Project Report



Stanley Consultants, Inc. 721 Government Street Baton Rouge TX 70802

Attention: Mr. Jacob M. Loeske, P.E., L.S.I.

Re: Geotechnical Engineering Report Bootlegger Road Improvements St. Tammany Parish, Louisiana

Dear Mr. Loeske:

APS Engineering and Testing, LLC is pleased to submit our Geotechnical Engineering Report for the above referenced project. The report includes the results of field and laboratory testing and recommendations for the pavement and bridge design and general site preparation as related to soils.

We appreciate the given opportunity to perform this Geotechnical study and look forward to continuing to participate during the design and construction phases of this project. If you have any questions pertaining to this report, or if we may be of further service, please contact our office.

Respectfully submitted, APS ENGINEERING AND TESTING, LLC







1.0 PROJECT INFORMATION

1.1 Project Authorization

APS Engineering and Testing has completed a subsurface exploration for the proposed Bootlegger Road improvement project in St. Tammany Parish, Louisiana. Authorization to proceed with the work was received from **Mr. Jacob Loeske** on February 3, 2017. Preliminary mill and overlay recommendations were submitted on March 31, 2017.

1.2 Project Description

The project site is located on highway LA-1085 in St. Tammany Parish, Louisiana, northeast of the intersection between Interstate 12 (I-12) and LA-1085. The LA-1085 road improvement section is about 4 miles in length, located between intersecting streets Turnpike Road (on the west side) and South Tyler Street (on the east side). The project will consist of performing mill and overlay work on the existing LA-1085 pavement, as well as the construction of a new bridge located just south of the intersection between LA-1085 and Weston Way. The east-bound and west-bound traffic data was provided by the client.

2.0 PURPOSE AND SCOPE OF SERVICES

The purpose of this study was to explore the subsurface conditions at the site to enable an evaluation of an acceptable foundation for the proposed structure as well as determining the existing pavement section thickness for mill and overlay recommendations. Two soil borings were drilled to a depth of 100 feet each in the proximity of the proposed new bridge, and three (3) soil borings to a depth of six (6) feet each (P-1 through P-3). Additionally, six (6) pavement cores were performed on the existing LA-1085 pavement down to a depth of two feet.

The scope of services also included conducting laboratory tests on selected samples recovered from the soil borings. These tests included visual description and classification, moisture content, liquid limit, plastic limit and unconfined compressive strength tests. Both field and laboratory testing procedures are briefly discussed in this report.

This report discusses the conditions of the existing subsoil materials at the site, and presents recommendations on the following:

- + Site preparation Recommendations;
- + Deep Foundation Recommendations for the proposed bridge structure;
- + Mill and Overlay Recommendations;
- + General Construction Recommendation; and
- + Comments regarding factors that will impact construction and performance of the proposed project.



The scope of geotechnical services did not include an environmental site assessment for determining the presence or absence of wetlands, hazardous or toxic materials in the soil, surface water, groundwater, or air on, below, or around the site. Any statements in this report or on the boring logs regarding odors, colors, and unusual or suspicious items or conditions are strictly for informational purposes.

3.0 SITE LOCATION AND DESCRIPTION

The project site is located on highway LA-1085 in St. Tammany Parish, Louisiana, northeast of the intersection between Interstate 12 (I-12) and LA-1085. The LA-1085 road improvement section is about 3.5 miles in length, located between intersecting streets Turnpike Road (on the west side) and South Tyler Street (on the east side). Approximate GPS coordinates of the project site are 30°28'1.27"N 90°10'46.04"W.

4.0 FIELD EXPLORATION

The field exploration, performed to evaluate the engineering characteristics of the foundation materials, included a reconnaissance visit to the project site by an APS representative, drilling the soil borings and recovering soil samples.

As previously mentioned, a total of two soil borings were drilled to a depth of 100 feet each in the proximity of the proposed new bridge, and three (3) soil borings to a depth of six (6) feet each (P-1 through P-3). Additionally, six (6) pavement cores were performed on the existing LA-1085 pavement down to a depth of two feet. The soil borings were located in the field by tape measurements/GPS coordinates that were referenced to physical elements/boundaries of the site. The Boring Location Plan, included in the Appendix, presents the approximate location of the soil borings.

5.0 DRILLING AND SAMPLING PROCEDURES

The borings were drilled with a track-mounted SIMCO 2800 drill rig, using dry auger and wet rotary drilling techniques to advance the boreholes. Undisturbed samples were continuously obtained from the ground surface to a depth of ten (10) feet, then at five-foot intervals to termination depth of each borings. They were obtained using thin-walled tube sampling procedures in general accordance with ASTM D-1587 Standard Practice for Thin-Walled Tube Sampling of Soils for Geotechnical Purposes. These samples were extruded in the field with a hydraulic ram, and were identified according to project number, boring number and depth, wrapped in aluminum foil and placed in plastic bags to preserve the natural moisture condition and transported to the laboratory in special containers to prevent disturbance.



For Cohesionless and semi-cohesive soils, Standard Penetration Tests (SPT) was performed at intervals to obtain standard penetration values of the soil. The standard penetration value (N) is defined as the number of blows of a 140-pound hammer, falling 30 inches, required to advance the split-barrel sampler 18 inches into the soil. To perform the test and obtain a sample, the sampler is lowered to the bottom of the previously cleaned drill rig and advanced by blows from the hammer. The number of blows is recorded for each of three successive increments of six inches penetration. The "N" value is obtained by adding the second and third incremental numbers. The results of the SPT indicate the relative density of cohesionless soils, and thereby provide a basis for estimating the relative strength and compressibility of the soil profile components. Soil samples were obtained utilizing a two inch O.D. split-barrel sampler in general accordance with procedures for "Penetration Test and Split-Barrel Sampling of Soils" (ASTM D1586).

6.0 LABORATORY TESTING PROGRAM

A laboratory testing program was conducted to determine pertinent engineering characteristics of the subsurface materials. This program included visual description and classification and determination of the moisture content (ASTM D2216 Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass) on all soil samples. Selected samples were subjected to ASTM D4318 Standard Test Methods for Liquid Limit, Plastic Limit and Plasticity Index of Soils, ASTM D422-63 Standard Test Method for Particle-Size Analysis of Soils, ASTM D1140-14 Standard Test Methods for determining the amount of material finer than 75-µm (No. 200) sieve in soils by washing and ASTM D2166 Standard Test Method for Unconfined Compressive Strength of Cohesive Soils. The results of these tests are found in the accompanying boring logs located in the Appendix.

7.0 SUBSURFACE CONDITIONS

7.1 Subsurface Materials

Bridge Borings

At the bridge borings, generally, beneath minimum of three (3) inches of asphalt, medium to very stiff lean and high plasticity clays (CL/CH) with clayey silt (ML) lenses were encountered to a depth of approximately 73 feet followed by dense to very dense sands down to the boring termination depth.

Pavement Borings

At the pavement borings, generally, beneath the 4 inches of asphalt, soft to medium silty clays (CL-ML) and fat clays (CH) were encountered to the termination depth of six (6) feet.



Pavement cores

The following table shows the thickness of the existing pavement sections at the six core locations:

TABLE 1.0				
Core Location	Gravel thickness (in)	Asphalt thickness (in)	Concrete thickness (in)	
C-1	6	5	-	
C-2	7	6	6	
C-3	6	4	-	
C-4	6	4	-	
C-5	7	4	-	
C-6	6	10	-	

TABLE 1.0

7.2 Groundwater

Ground water was encountered at a minimum depth of 20 feet from the existing ground surface, during our field exploration. Wet rotary drilling techniques were used from ten (10) feet to the termination depth of respective borings. It should be noted that the groundwater conditions are likely to change due to topography, permeability, weather, and other soil and terrain properties. Therefore, we recommend that the contractor determine the actual groundwater levels at the site at the time of the construction activities.

8.0 DISCUSSION

Upon review of the existing subsoil conditions and laboratory tests results, we consider that the proposed project is feasible from a geotechnical point of view, provided that the included recommendations are correctly interpreted and applied.

Generally, the encountered subsoil materials provided good strength parameters; this is based on unconfined compression strength test results. Based on these conditions and the provided design loads, the proposed bridge can be supported on a deep foundation system.

Please review the following sections for further information on the corresponding site and foundation recommendations. Site development recommendations are applicable only for shallow footings.



9.0 GEOTECHNICAL RECOMMENDATIONS

9.1 Site Development Recommendations

9.1.1 Site Preparation

Prior to the development of any structure or fill deposit, the complete earthwork area must be properly cleaned. The cleaning activities shall include the removal of all surface vegetation, debris and any foreign matter present on the site.

The actual removal depth of topsoil shall be determined in the field by the Geotechnical Engineer or a representative. Please note that the stripped materials can only be used in landscaping, but not at any structural area.

9.1.2 Proof Rolling

Upon completion of the stripping activities, the exposed areas shall be properly proof rolled in order to prepare the natural terrain to receive the design structural fill and traffic loads. The proof roll consists of compacting the exposed surface with a 20-ton loaded dump truck. Surface soils that are observed to rut or deflect excessively under the truck load should be undercut and replaced with the proper structural fill. These activities should be performed during a period of dry weather and should be supervised by a Geotechnical Engineer or a representative.

9.1.3 Structural Fill Materials

After subgrade preparation and observation has been completed, structural fill placement may begin. The first layer of structural fill should be placed in a relatively uniform horizontal lift and be adequately keyed into the properly prepared subgrade soils. The structural fill should consist of lean clays, sandy lean clays (CL) or clayey sands (SC) having the following recommended material properties:

- a. Percent Passing U.S. Sieve #200: 50 percent minimum
- b. Liquid Limit: 40 maximum
- c. Plasticity Index: 10 to 20 maximum
- d. Inert Material (Non-Expansive)
- e. Free of Organics
- f. Maximum Particle Size: 2-in

This material must be certified and approved by the Geotechnical Engineer prior to its use. In case this material is not available locally or economical for project, it is recommended to consider the following material as an alternative.



An alternative structural fill material could consist of "clean" sand or pumped sand having less than 10 percent fines passing the No. 200 Sieve. It should be compacted to at least 95 percent of Maximum Dry Density at Optimum Moisture Content according to ASTM D-698. In-place density measurements should be taken to assure that this degree of compaction is achieved. This material must be certified and approved by the Geotechnical Engineer prior to its use.

9.1.4 Structural Fill Deposit Construction

After all surface preparation and observation has been completed, the structural fill activities may begin. These activities must be performed in a sequential order where lower elevations must be worked before higher ones. The structural fill shall be deposited in lifts of eight (8) inches of loose material. Each lift shall be compacted and certified by the Geotechnical Engineer or a representative prior to placement of other lifts. The passing criteria shall be a 95% of the maximum dry density as determined by ASTM D-698, Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft³ (600 kN-m/m³)), and a moisture content between one (1) below and three (3) above percentages of the optimum moisture content. If water must be added, it should be uniformly applied and thoroughly mixed into the soil by disking or scarifying. As a guideline, it is recommended that field density tests be performed at a frequency of not less than one test per 2,500 square feet.

It is important to maintain the structural fill thickness as uniform as possible. Uneven fill thicknesses under a structure may cause differential soil responses to the applied loads which can produce cracking, settling, or tilting of the structure. Uniform fill areas shall consider the footprint of the structure plus a five (5) feet strip around its perimeter.

Fill slopes shall be maintained at a maximum 2 Horizontal: 1 Vertical steepness. The runoff of water across the faces of the slopes shall be avoided by appropriate drainage ways. In addition, appropriate drainage ways shall be maintained at all earthwork surface areas in order to not affect compaction.

9.2 Deep Foundation Recommendations

Driven foundation systems were evaluated for the proposed bridge structure. APS has evaluated 16inch square precast concrete piles to support these structures. The skin friction of the top 20 feet of soil was neglected in our analysis. Single piles were modeled with the Driven 1.2 software using deep soil borings from the project site. We recommend that all the piles in a bent be embedded to the same elevation. Structural fill placement was not taken into consideration in determining the axial pile capacities. If more than two (2) feet of structural fill will be placed at the proposed structure, APS should be contacted to revise our calculations.

Proposed Structure	Strengt Bent No. Load ¹		AASHTO LRFD Resistance Factor	Factored Load (Tons)	Pile Length (feet)
Structure	(Tons)	16-inch			
Bridge	1	130	0.50	260	95
	2	187	0.50	374	95
	3	187	0.50	374	95
	4	130	0.50	260	95

TABLE 2.0

1. Provided by the Structural Engineer

TABLE 3.0					
Proposed Structure	Bent No. Strength (Tons)	Strength Load ¹	rrength Load ¹ AASHTO LRFD Resistance	Factored Load	Pile Length (feet)
		Factor ²	(Tons)	16-inch	
Bridge	1	130	0.80	163	80
	2	187	0.80	234	80
	3	187	0.80	234	80
	4	130	0.80	163	80

1. Provided by the Structural Engineer

2. A Static Load Test must be performed to use this factor

Medium dense to dense clayey sands and poorly graded sands may create difficult pile driving conditions, that can be encountered from 75 to 100 feet. A WEAP analysis must be performed to evaluate conditions of pile drivability once the contractor selects the actual pile driving equipment (hammer type). It is not recommended to purchase production piles prior to determining the pile drivability.

9.2.1 Settlement

The estimated settlement of individual piles properly driven to the design depths and loaded to the design capacities will be on the order of one (1) inch (with No Structural fill placement taken into consideration).

9.2.2 Group Effects

A group of piles subjected to vertical loads may not necessarily have the same capacity as the sum of the individual pile capacities. For axially loaded piles, published results indicate that the ratio of capacity per pile in a group to that of a single isolated pile typically ranges from 0.5 to 1.0. This efficiency factor depends on the spacing or distance between each pile. In planning pile groups, a minimum center-to-center spacing of four (4) diameters (4D is the diameter or width) is recommended to avoid a reduction in group capacity. Group action should be checked after the actual pile spacing is determined.



9.2.3 Lateral Capacity

For deep foundations, the lateral loads are resisted by the soil as well as the rigidity of the pile. Analyses can be performed by methods ranging from chart solutions to finite difference methods. It is recommended that once the pile type, length and group dimensions are determined, our office be contacted to perform lateral load analysis for the proposed project.

9.2.4 Pile Load Test

It is recommended that pile capacities be verified by field load test. It is recommended that at least one test pile of each type of pile to be used on the site be driven in the proposed foundation area and tested in compression as outlined by ASTM D 1143. The pile load test(s) should be performed under the guidance of the geotechnical engineer so that the data may be interpreted and the recommended pile capacity adjusted, if necessary, according to the load test results. <u>APS highly recommends that the static load test must be performed prior to purchasing and driving any production piles.</u>

9.2.5 Driven Pile Installation

Piles at this site will derive their capacity predominantly from frictional resistance between the piles and the surrounding soils. We recommend all pile driving operations be inspected by a qualified geotechnical inspector. Records of driving resistance versus depth, tip evaluation of piles, driving equipment, pile size and length, etc. should be permanently kept. The hammer selection is the responsibility of the contractor and should be adequately large enough to reach plan tip elevations and develop the required capacities.

The piles should not be jetted or spudded into place. Piles in large groups should be driven from the center outward. Any piles which have heaved ¼ inches during driving of subsequent piles should be re-driven to their original final resistance or their original embedment if originally driven to full penetration.

9.2.6 Pile Driving Monitoring

Records of pile size and length, driving equipment, driving resistance versus depth, tip evaluation of piles, etc. shall be permanently kept.

Sometimes premature refusal occurs due to poor performance of the hammer rather than from soil resistance. Any changes in hammer blow counts shall be carefully examined before making any decisions about the pile penetration.

Since testing and inspection services are within APS's scope of work, we recommend that our firm be retained to assist you in establishing final pile lengths, maintain vibration and driving records of all piles installed.



9.2.7 Pile Driving Resistance

To determine the driving characteristics, a few probe piles should be driven beneath the proposed structures, preferably in the vicinity of the borings. Probe piles will become working piles, and must be accurately located in accordance with the project's construction drawings. Exact driving resistance recommendations should be determined based on the actual pile driving equipment selected by the contractor and the driving results of the probe piles.

10. PAVEMENT RECOMMENDATIONS

Pavement Sections

Actual traffic type and frequency were provided by the client, and data was used to estimate the Equivalent Single Axle Loads (ESALs) for the pavement rehabilitation evaluation. Mill and overlay thickness was determined using the AASHTO method. Based on the pavement core results, the existing asphalt thickness varies between 4 and 10 inches. Our recommendations are based on a pavement design life of 20 years and based on the specific wheel loads, axle configurations and frequencies provided by the client.

Proper finishing of concrete pavement requires the use of appropriate construction joints to reduce cracking. Construction joints shall be designed in accordance with the current Portland Cement Association and the American Concrete Institute guidelines. Joints should be sealed to reduce the potential for water infiltration into the supporting soils. Concrete compressive strength shall meet 3,000 psi at 28 days with a maximum slump of 4-inches.

A CBR of 3.0 and a Modulus of Subgrade Reaction (k) of 90 pci were assigned to the near surface well compacted and stable subgrade soils for the pavement evaluation. The recommended pavement mill and overlay thicknesses presented below were based on the assumed parameters at the site and pavement core results:

Core Location	Existing gravel and asphalt/concrete thickness	Mill and Overlay recommendation	
C-1	6" Gravel, 5" Asphalt	Mill 2 inches and overlay a minimum of 5 inches	
C-2	7" Gravel, 6" Asphalt ,plus 6" Concrete	Mill 2 inches and overlay a minimum of 2 inches	
C-3	6" Gravel, 4" Asphalt	Mill 2 inches and overlay a minimum of 6 inches	
C-4	6" Gravel, 4" Asphalt	Mill 2 inches and overlay a minimum of 6 inches	

TABLE 4.0



Core Location	Existing gravel and asphalt/concrete thickness	Mill and Overlay recommendation
C-5	7" Gravel, 4" Asphalt	Mill 2 inches and overlay a minimum of 6 inches
C-6	6" Gravel, 10" Asphalt	Mill 2 inches and overlay a minimum of 2 inches

TABLE 5.0

FLEXIBLE PAVEMENT			
Dovement Metoviels	Minimum Thickness, Inches		
Pavement Materials	Sidewalk		
Asphaltic Concrete	3		
Well Compacted and Stable Subgrade/Fill	12		

TABLE 6.0

Average Daily Traffic (ADT)	10,000
% Trucks	2.0%
% Growth	2.0
Directional Distribution	50%
Design Lane Distribution	100%
Design Life	20 years

BOOTLEGGER ROAD - SECTION DESIGN DATA				
	MILL &	FULL		
	OVERLAY	RECONSTRUCTION		
2017 ADT		10,000		
2037 ADT		12,000		
TWO WAY TRAFFIC (ADT)	10,000			
PERFORMANCE PERIOD (YEARS)	10	20		
18-KIP ESALS	1,408,900	2,817,800		
SOIL RESILIENT MODULUS (PSI)	3,000	3,650		
STRUCTURAL NUMBER REQUIRED (INCHES)	4.65	4.65		
STRUCTURAL NUMBER PROVIDED, INCHES (PER LAYER)				
1-SUPERPAVE AC WEARING COURSE (LEVEL 2F)	2.52	1.68		
2-SUPERPAVE AC BINDER COURSE (LEVEL 2)	0.50	1.60		
3-CLASS II BASE COURSE	0.72	1.44		
STRUCTURAL NUMBER PROVIDED (TOTAL INCHES)	3.74	4.70		

FLEXIBLE PAVEMENT			
	Minimum Thickness (Inches)		
Pavement Materials	Bridge Approach Pavement		
Asphaltic Concrete	8		
Aggregate Base Course	12		
Well Compacted & Stable Subgrade or Fill	12		

11 HEADWALL RECOMMENDATIONS

Based on the information provided by the client, wingwalls will be approximately four feet high and be supported on subsurface soils present at a depth of five to 10 feet from the ground surface (pavement). An allowable bearing capacity of 1,000 psf can be used for these soils at this depth.

Reinforced concrete, below grade walls of the Wing-wall structure will be subjected to unbalanced earth pressures and should be designed for at-rest earth pressures at least equal to those indicated in tables below. Earth pressures will be influenced by structural design of the walls, condition of wall restraint, methods of construction and/or compaction and strength of materials being restrained. The at-rest condition should be used where no wall movement is permitted. The recommended design lateral earth pressures do not include a factor of safety.

	TABLE 9.0											
Material	Soil Moist Equivalent Unit Fluid Weight, Y Pressure		Undrained Cohesion, C _d And Friction	Coefficient Of Earth Pressure								
	(РСТ)	(рст)	Angle, F	Ко								
Sand	126	63	0 psf, 30°	0.5								
Native soils	120	120	1,500 psf, 0°	1.0								

Lateral Earth Pressure, psf = Ko x (q + Y H)



AT-REST CONDITION (BELOW GROUND WATER TABLE)

		TABL	E 10.0		
Material	Soil Moist Unit Weight (Pcf)	Soil Saturated Unit Weight Ƴsat (pcf)	Equivalent Fluid Pressure (pcf)	Undrained Cohesion, Cd And Friction	Coefficient Of Earth Pressure
				Angle, F	Ко
Sand	120	125	94	0 psf, 30°	0.5
Native soils	120	125	125	1,500 psf, 0°	1.0

Lateral Earth pressure, psf = (Ko x q) + (Υ x H_w x Ko) + [(Υ _{sat} - Υ _w) x (H - H_w) x K_o] + [Υ _w x (H - H_w)]

12.0 CONSTRUCTION CONSIDERATIONS

12.1 Observation and Testing

The preceding recommendations require a close supervision of the Geotechnical Engineer or representative; therefore, it is recommended that APS be retained to provide observation and testing for the complete duration of all earthwork and foundation activities for this project. APS cannot accept responsibility for any conditions deviated from those described in this report, nor for the performance of the foundation if not engaged to provide construction observation and testing.

12.2 Moisture Sensitive Soils/Weather Related Concerns

Most of the subsurface materials encountered at this site are expected to be sensitive to disturbances caused by changes in moisture content. During wet weather periods, the increment of the moisture content of the soil may cause a significant reduction of the soil strength and support capabilities. Furthermore, soils that become wet may be slow to dry, thus significantly retarding the progress of grading and compaction activities. For these reasons, it will be advantageous to perform earthwork and foundation construction activities during dry weather.

12.3 Excavations Regulations

In the Federal Register, Volume 54, No. 209 (October 1989), the United States Department of Labor, Occupational Safety and Health Administration (OSHA) amended its "Construction Standards for Excavations, 29 CFR, part 1926, Subpart P". This document was issued to better insure the safety of workmen entering trenches or excavations. It is mandated, by this federal regulation, that excavations, whether they be utility trenches, basement excavations or footing excavations, be constructed in accordance with the new OSHA guidelines.



The contractor is solely responsible for designing and constructing stable, temporary excavations and shall shore, slope, or bench the sides of the excavations as required to maintain stability of both the excavation sides and bottom. The contractor's "responsible person", as defined in 29 CFR Part 1926, should evaluate the soil exposed in the excavations as part of the contractor's safety procedures. In no case should slope height, slope inclination, or excavation depth, including utility trench excavation depth, exceed those specified in local, state, and federal safety regulations.

We are providing this information solely as a service to our client. APS does not assume responsibility for construction site safety or the contractor's or other parties' compliance with local, state, and federal safety or other regulations.

13.0 REPORT LIMITATIONS

The analyses and recommendations presented in this report are based on the existing field conditions at the time of the investigation. Furthermore, they are based on the assumption that the exploratory borings are a representation of the subsoil conditions throughout the site. Please note that variations in the subsoil conditions may occur between and beyond borings. If variations in those conditions are encountered during construction, APS shall be notified immediately in order to assess the situation, confirm the recommendations included in this report, or modify them according to their own judgment. If APS is not notified of such variations, APS will not be responsible for the impact of those variations on the project.

Furthermore, this report is based on the design considerations presently known to us. Project designers must be aware of this situation to check if any important design parameter has been overlook or requires additional clarification. If the nature of the project should change, the recommendations given in this report shall be re-evaluated. If APS is not notified of such changes, APS will not be responsible for the impact of those changes on the project.

APS shall be retained for the review of final design drawings and specifications in order to ascertain whether their recommendations have been correctly interpreted and implemented and to confirm or modify them. APS is not responsible for the adequacy of recommendations if they do not inspect the construction. The only warranty regarding our services is that the findings, recommendations, specifications, or professional advice contained herein have been made in accordance with the generally accepted professional geotechnical engineering practices in the local area. No other warranties are implied or expressed.

This report has been prepared for the exclusive use of **Stanley Consultants**, **Inc.**, and their design/construction team associated to this specific project.

APPENDIX



HLd30 Fee o	ELEVATION	GRAPHIC	SOIL TYPE S AND COLOR	WET DENSITY	MOISTURE CONTENT	LIQUID	PLASTICITY INDEX	% PASSING #200	SPT or UU	FAILURE MODE/ SPT TERMINATION	SAMPLE TYPE NUMBER	DRILL RIG AND EQUIPI DRILL RIG MODEL: SIMC0 2800 DRILLING METHOD. WET METHOD HOLE DIAMETER: 4 Inches SPT HAMMER / ETRAUTO	MENT	HLd30 Feet o	ELEVATION	GRAPHIC	SOIL TYPE AND COLOR	WET DENSITY	MOISTURE CONTENT	LIMIT	INDEX % PASSING #2000	SPT or •UU	FAILURE MODE/	SPI IEKMINATION SAMPLE TYPE	DR DRILL RIG DRILLING HOLE DIAU SPT HAMM	ILL RIG AND E MODEL: SIMCO METHOD: WET N VETER: 4 Inchr VET ETR: 4 Inchr VET AUTO PL NM 30 40 50	EQUIPN 2 2800 METHOD es 60 7)	MENT
-			Three (3) Inches of Asphalt Stiff, gray, FAT CLAY, (CH)	126	25	51	33		1.16@2.00	SL.	1	- - - - 0	-	-			Three (3) Inches of Asphalt . Medium stiff, tan gray, FAT CLAY, (CH)	117	41	77	52	0.53@2.0	10 60 1	s.	- - 1	⊢−−		
10			Very stiff, tan gray, FAT CLAY, (CH)	127	22	66	48		2.10@4.00	SL.	2			10			Stiff, tan gray, FAT CLAY, (CH)	129	23	51 :	15	1.50@4.0	10 60 5	s.	2	əi		
-			with Fe nodules and traces of	127	22						3			-				125	23	50 :	84	1.47@6.0	10 60 5	s.	3	ei		
20			Stiff, tan gray, FAT CLAY, (CH)	115	39	91	63		1.24@8.00	60 S.	4	- - -		20				122	34			s			4			
-				112	44						5			-			Medium stiff, gray, LEAN CLAY, (CL) -with sand	130	28	31	0 49	.9 0.74@10.	DO SL		5 - F	-01		
30			Medium stiff, gray, FAT CLAY, (CH)	100	62	98	72		0.56@12.00	60 S.	6	-		30			-with sand	127	26						6			
-			Stiff, gray, LEAN CLAY, (CL) with sand pockets	127	24	40	23		1.89@14.00	SL.	7	- - + 0 1		-			Stiff, tan gray, FAT CLAY, (CH)	112	42 1	04	'6	1.62@14.	00 60 9	s.	7	н — о —		
40				126	25	41	21		2.03@16.00	60 S.	8	- + 0 - 1		40			Very stiff, gray, LEAN CLAY, (CL)	127	24						8			
-			Medium dense, gray, Clayey Silt, (ML) -with sand	128	29			51.3	1.75@18.00	60 S.	9			-				125	25	35	7	2.77@18.	00 60 9	а.	9	ଡ଼──┥		
50			Gray, LEAN CLAY, (CL) -with silt	119	30	36	17				10		s. 	50			Loose, gray, Clayey Silt, (ML) -with sand lenses	122	28						10			
-			Stiff, gray, FAT CLAY, (CH)	113	41	73	48		1.65@22.00	60 S.	11	-	-	-			Stiff, tan gray, LEAN CLAY, (CL) -with silt	129	35	45 3	24	1.91@22.	00 60 9	5.		0_ 		
_ 60 -			Stiff, tan gray, FAT CLAY, (CH)	115	41	85	60		2.06@24.00	60 S.	12	- 1-0		60			Very stiff, gray, FAT CLAY, (CH)	111	43	90 (12	2.40@24.	00 60 8	ŝ.	12	⊢ 0		1
-			-with sand	113	37	77	54		1 00 000 00		13			-			with sand	127	25	46 2	:6	2.63@26.	DO SL		13			· · · ·
-			Dense, tan gray, CLAYEY SAND, (SC)	-	21			24.2	8-14-18	00 8.				_ 70			Gray, CLAYEY SAND, (SC)	108	37	// :	21	1.72@28.	0 60 3	».	14			•
80			Dense, gray, POORLY GRADED SAND WITH CLAY, (SP-SC)	_	24			6.7	(32) 16-19-22	4	X 16	-		80			Dense, tan, CLAYEY SAND, (SC)		21			20-23-2	5 4	X	16			
-					19				22-26-30 (56)	4	17			-			Dense, gray, POORLY GRADED	-	16		8.	(48) 3 17-20-2: (43)	3 4		17			
90			Very dense, gray, POORLY GRADED SAND, (SP)		19			3.0	26-28-43 (71)	4	X 18	-		90					18			19-23-20 (49)	4	×	18			
-			Very dense, gray, POORLY GRADED SAND WITH CLAY, (SP-SC)	-	19			9.0	26-28-43 (71)	4	X 19			-			Very dense, gray, POORLY GRADED SAND WITH CLAY, (SP-SC)	-	19		10.	6 19-39-5((89)	4	\square	19			
- 100			Very dense, gray, POORLY GRADED SAND. (SP) Bottom of hole at 100 feet Backfilled with grout upon completion.	-	20			4.7	26-39-50 (89)	_4	X 20			- - - 100			Bottom of hole at 100 feet Backfilled with grout upon completion.		18	_	7.	3 19-36-50 (86)	4	X	20			
-														-														
BOF	RING	NO. B-1		STA	ATION:				1		w			BORI	ING NO) Э. В-2		STA	ATION:					<u> </u>	WATER LEVEL			
LAT		: 30.46 DE: -90	13664	OFF	SET:	EN 3	174/20	17			BA					30.46	13637	OFF	SET:	1 2/22	2017			_				
LRS	S ID:		FIELD BOOK	STR	RUCTU	RE NC).:				DF	ILLER: VG		LRSI	ID:	00	FIELD BOOK:	STR	RUCTURE	NO.:	201/				DRILLER: V	G	·	
		STAND	ARD ABBREVIATIONS & DEFINITION	ONS								SOIL PROPERTIES					MIS	CELLA									co	RRELATIC
	ASPHAL1	r	СН		N.P.	= No	on-Plast	ic			WETP	S = SOIL TYPE nomenda	ature is based on	ASTM D	D 2487	mined		= L	ocation and	d Identifi	cation of	thin-walled tub	e sample	e, AAS⊦	ITO T 207		RESI	STANCE A
	CL.		ML		ORG	. = Or	ganic			мо	DISTUR	CONTENT = Moisture Content of in	in-place soil, expre	ET CU. TI. ESSED as	s a perc	entage	of the dry weight	= L	ocation and	d Identifi	cation of	thin-walled tub	e sample	e, AAS⊦	ITO T 207,	SOIL		DESIGN
2222	SC		SP-SC								LIQUID	LIMIT & = Atterberg limits and in	nined by DOTD T ndices, DOTD TR	.x 403, N . 428	wethod	в.	D-3	v = L	nth a portio ocation and	n of the I Identifi	sample s cation of	aved for conso SPT sample, A	ASHTO	esting T 206		SAND) 52	VER
	SP				M.S	<u>FA</u> = Multi	ILURE	MODE: ar SL.	= Slump	P	LASTIC	TY INDEX T = Standard Penetration	n Test, AASHTO 1	í 206, nu	umber d	of blows	per each 6 inch	= L	ocation an	Identifi	cation of	sample recove	red usin	ıg an av	ger as per	AND	ELATIO	MED
					S/S V.S.	= Slick = Verti	ensider ical She	ar 60 S	. = Yield . = Shear And	e	ι	increment, unless am U = Unconsolidated Undr	nount of penetration rained triaxial test.	in is sho , AASHT	own FO T 29	6, comi	pressive strength 💀 G-3	م = 0	STM D145 Grab Sampl	2 e, unabl	e to reco	ver undisturbe	sample	tor stre	ngth testino	SILT	~ ~ ~	VER
					SPT	TERMI	NATION	AASH	TO T 206			(tons per sq. ft.), of on = Soil cohesion (tons r	ne specimen confi per sg. ft)	ned at n	oted pr	essure	(pounds per sq. in.)	and = N	d material r	tained	or classi	fication.	estine c	t class?	ication		č	VE
					1 =	7.2.1 - 5 7.2.2 - 1	100 Blow	ns Within ws ⊺ota	A 6" Interval			Soil angle of internal	friction (degrees)	AAcut	TO T 20	6 three		= 0	isturbed sa	mple re	covered	with thin-walled	tube sa	mpler.	outron f.	CLAY	, Is TENC	ME
					3 =	1.2.3 - 1	No Advi	anceme	nt for 10 Blows			a = Unconsolidated Undr	aneo triaxial test,	MASHT	0129	o, three	specimens, (c - •). ¥	= V	vater Table	depth b	elow gro	und surface at	ume of c	rilling			NSI	

SAIRAM E License PROFESSIO

REVIATIONS & DEFINITIONS		SOIL PR	- SOU TYPE pomonology is board on ASTM D 2497	MISC			CORF	RELATION OF PENI ANCE AND SOIL P	ETRATION ROPERTIES
	N.P. = Non-Plastic ORG. = Organic	WET DENSITY MOISTURE CONTENT	 Wet density of in-place soil, (pounds per cu. ft.) determined by AASHTO T 208. Moisture Content of in-place soil, expressed as a percentage of the dry weight 	C -3	 Location and identification of thin-walled tube sample, AASHIOT 207 Location and Identification of thin-walled tube sample, AASHTOT 207, 	SOIL	D	DESIGNATION	"N" (blows per ft.)
SP-SC			of the soil, (%), determined by DOTD TR 403, Method B.		with a portion of the sample saved for consolidation testing			VERY LOOSE	LESS THAN 4
		LIQUID LIMIT &	= Atterberg limits and indices, DOTD TR 428	X P-3	= Location and Identification of SPT sample, AASHTO T 206	SAND	۳۶	LOOSE	4 - 10
	FAILURE MODE:	PLASTICITY INDEX		-		AND	VSI	MEDIUM DENSE	10 - 30
	M.S. = Multiple Shear SL. = Slump	SPT	= Standard Penetration Test, AASHTO T 206, number of blows per each 6 inch	G-3	= Location and Identification of sample recovered using an auger as per	SILT	DEL	DENSE	30 - 50
	S/S = Slickensides YLD. = Yield		increment, unless amount of penetration is shown	Flax	ASTM D1452		-	VERY DENSE	OVER 50
	V.S. = Vertical Shear 60 S. = Shear Angle	UU	= Unconsolidated Undrained triaxial test, AASH10 1 296, compressive strength		= Grab Sample, unable to recover undisturbed sample for strength testing			VERY SOFT	LESS THAN 2
	SPT TERMINATION, AASHTO T 206	0	(tons per sq. tt.), of one specimen contined at noted pressure (pounds per sq. in.)	NO DEOV	and material retained for classification.		7	SOFT	2.4
	1 = 7.2.1 - 50 Blows Within A 6" Interval		= Soli conesion (tons per sq. tt.)	NU RECV.	No Recovery, Unable to recover sample for testing or classification.		EN C	MEDILIM STIEF	2-4
	2 = 7.2.2 - 100 Blows Total	•	 Soli angle of internal inclion (degrees) Incomposidated Underined triavial test, AACHTO T 206, three ensurements (c., t). 		= Disturbed sample recovered with thin-walled tube sampler.	CLAY	LSI	OTIES	4-0
	3 = 7.2.3 - No Advancement for 10 Blows		 Enconsolidated drained direct elses test. AASH 10 T 296, (mee specimens, (c - +). 	¥	= vvater Table depth below ground surface at time of drilling		SNO	SHFF	8-15
	4 = 7.2.4 - Sampler Driven the Entire 18		- Consolidated dramed direct shear test, AASH 10 1 235, (C - V).	*	= vvater i able deptri below ground surface aπer drilling as noted		8	VERY STIFF	15 - 30
	5 - Non-stanuaru	-	- nydrometer test performed	EIR=	= Energy Transfer Ratio determined according to ASTM D4633			HARD	OVER 30

	s	HEE	т	_		
		ST. TAMMANY	ER I			
		PAKISH	CONTRC	SECTION	STATE	PROJEC
			SP	SE	4/4/2017	
	ESIGNED	HECKED	ETAILED	HECKED	ATE	HEET
						BY
						REVISION DESCRIPTION
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F LOUISIANIII		A State of the sta				
EDDANAPUDI No. 35129 ONAL ENGINEER N GINEER 5 25/17		GEOTECHNICAL				Bootlegger Road Improvements
THES SIGNATURE AND SEAL IS AFFICED TO THIS DRAWING AS CORTIFICATION THAT THE LABORATORY TESTING AND ANULYSIS WAS PERFORMED ACCORDING TO THE USTED PRACEDURES NO DESIGN COMPUTATIONS WERE PERFORMED OR REVIEWED BY ME		AT DAMAGE				





BORING LOG

BORING NO.: P-1 **PROJECT NO.:** APS1702-G009 **PROJECT:** Bootlegger Road Improvements **METHOD:** AUGER **PROJECT LOCATION:** St. Tammany Parish, LA **BORING ELEVATION:** Not Surveyed BORING LOCATION: N-3639.94 E4023193.7 **DATE COMPLETED:** 03/24/2017 **DATE DRILLED:** 3/24/2017 **WATER LEVEL DATE:** 03/24/2017 WATER LEVEL: Not Encountered DRILLER: VG **GEOL/ENGR:** SE/SA DEPTH (FEET) SAMPLE Moisture Content Symbol LL ΡI MATERIAL CLASSIFICATION (%) Gray Lean Clay (CL) -with sand @ 0'-1' : -200 = 60.6 % 21 Tan and Gray Lean Clay (CL) -with Fe nodules 21 43 27 2 Reddish Tan and Gray Fat Clay (CH) 27 3 26 Reddish Tan and Gray Lean Clay (CL) 22 5 20 6 Boring terminated @ 6.0 feet. Boring backfilled with cuttings upon completion. 8 10 COMMENTS: Auger Cuttings

BORING LOG

BORING NO.: P-2 **PROJECT NO.:** APS1702-G009 **PROJECT:** Bootlegger Road Improvements **METHOD:** AUGER **PROJECT LOCATION:** St. Tammany Parish, LA **BORING ELEVATION:** Not Surveyed BORING LOCATION: N-4053.68 E4025175.4 **DATE COMPLETED:** 03/24/2017 **DATE DRILLED:** 3/24/2017 **WATER LEVEL DATE:** 03/24/2017 WATER LEVEL: Not Encountered DRILLER: VG **GEOL/ENGR:** SE/SA DEPTH (FEET) SAMPLE Moisture Content Symbol LL ΡI MATERIAL CLASSIFICATION (%) Reddish Tan Silty Clay (CL-ML) -with sand @ 0'-1' : -200 = 29.4 % 20 6 21 Reddish Tan and Gray Lean Clay (CL) 23 28 8 2 Tan and Gray Fat Clay (CH) 25 3 27 Reddish Tan and Gray Fat Clay (CH) 37 5 36 6 Boring terminated @ 6.0 feet. Boring backfilled with cuttings upon completion. 8 10 COMMENTS: Auger Cuttings

APS Design and Testing, LLC

BORING LOG

BORING NO.: P-3 **PROJECT NO.:** APS1702-G009 **PROJECT:** Bootlegger Road Improvements **METHOD:** AUGER **PROJECT LOCATION:** St. Tammany Parish, LA **BORING ELEVATION:** Not Surveyed **BORING LOCATION:** N-5737.87 E4026416.99 **DATE COMPLETED:** 03/24/2017 **DATE DRILLED:** 3/24/2017 **WATER LEVEL DATE:** 03/24/2017 WATER LEVEL: Not Encountered DRILLER: VG **GEOL/ENGR:** SE/SA DEPTH (FEET) SAMPLE Moisture Content Symbol LL ΡI MATERIAL CLASSIFICATION (%) Gray Lean Clay (CL) -with roots 23 15 34 Gray and Reddish Tan Lean Clay (CL) 29 2 Reddish Tan and Gray Lean Clay (CL) 20 3 Gray Fat Clay (CH) 30 30 5 32 6 Boring terminated @ 6.0 feet. Boring backfilled with cuttings upon completion. 8 10 COMMENTS: Auger Cuttings

APS Design and Testing, LLC

Section 03 COQUILLE AREA DRAINAGE IMPROVEMENTS

Summary of Drainage Structures

STRUCTURE	STATION	SIDE OF		STORM DRAIN	STORM D	RAIN PIPE	CATCH	BASIN	
NO.	STATION	C/L	DESCRIPTION	15" (FFFT)	42" (FFFT)	48" (FFFT)	CB-02 (FACH)	CB-04 (FACH)	
STA. ALONG	BOOTLEGGER	RD		13 (1221)		10 (1221)			CO. 1D.
100	108+66	LT.	RCPA, 42 in. x 24 ft.		24				3.4
101	108+78	LT.	CATCH BASIN				1		1.4
102	109+34	LT.	RCPA, 48 in. x 112 ft.			112			16.6
103	109+90	LT.	CATCH BASIN				1		1.4
103A	109+90	LT.	RPVCP, 15 in. x 11 ft.	11					0.9
103B	109+90	LT.	CATCH BASIN					1	0.7
104	110+60	LT.	RCPA, 48 in. x 140 ft.			140			20.7
105	111+30	LT.	CATCH BASIN				1		1.4
105A	111+30	LT.	RPVCP, 15 in. x 11 ft.	11					0.9
105B	111+29	LT.	CATCH BASIN					1	0.7
106	111+70	LT.	RCPA, 48 in. x 80 ft.			80			11.9
107	112+26	LT.	CATCH BASIN				1		1.4
107A	112+26	LT.	RPVCP, 15 in. x 11 ft.	11					0.9
107B	111+26	LT.	CATCH BASIN					1	0.7
108	112+95	LT.	RCPA, 48 in. x 170 ft.			170			25.2
109	113+80	LT.	CATCH BASIN				1		1.4
110	114+66	LT.	RCPA, 48 in. x 40 ft.			40			5.9
111	114+86	LT.	CATCH BASIN				1		1.4
112	114+97	LT.	RCPA, 48 in. x 24 ft.			24			3.6
GENERAL TO	TAL			33	24	566	6	3	100.4

Section 04 Revised

LOUISIANA UNIFORM PUBLIC WORK BID FORM

TO: St. Tammany Parish Government 21454 Koop Dr., Suite 2F Mandeville, La 70471

BID FOR:Bootlegger Road Improvements Bid #300-00-18-66-2

(Owner to provide name and address of owner)

(Owner to provide name of project and other identifying information.)

The undersigned bidder hereby declares and represents that she/he; a) has carefully examined and understands the Bidding Documents, b) has not received, relied on, or based his bid on any verbal instructions contrary to the Bidding Documents or any addenda, c) has personally inspected and is familiar with the project site, and hereby proposes to provide all labor, materials, tools, appliances and facilities as required to perform, in a workmanlike manner, all work and services for the construction and completion of the referenced project, all in strict accordance with the Bidding Documents prepared by:_ St. Tammany Parish Procurement (Owner to provide name of entity preparing bidding documents.) _____ and dated: ______ December 3, 2018

Bidders must acknowledge all addenda. The Bidder acknowledges receipt of the following ADDENDA: (Enter the number the Designer has assigned to each of the addenda that the Bidder is acknowledging) _

TOTAL BASE BID: For all work required by the Bidding Documents (including any and all unit prices designated "Base Bid" * but not alternates) the sum of:

Dollars	(\$		
---------	-----	--	--

)

ALTERNATES: For any and all work required by the Bidding Documents for Alternates including any and all unit prices designated as alternates in the unit price description.

Alternate No. 1 for the lump sum of:

	Dollars (\$)
Alternate No. 2 (N/A) for the lump sum of:			
<u>N/A</u>	Dollars (\$	N/A)
Alternate No. 3 (N/A) for the lump sum of:			
<u>N/A</u>	Dollars (\$	N/A)
NAME OF BIDDER:			
ADDRESS OF BIDDER:			
LOUISIANA CONTRACTOR'S LICENSE NUMBER:			
NAME OF AUTHORIZED SIGNATORY OF BIDDER:			
TITLE OF AUTHORIZED SIGNATORY OF BIDDER:			

SIGNATURE OF AUTHORIZED SIGNATORY OF BIDDER **:

DATE:

THE FOLLOWING ITEMS ARE TO BE INCLUDED WITH THE SUBMISSION OF THIS LOUISIANA **UNIFORM PUBLIC WORK BID FORM:**

* The Unit Price Form shall be used if the contract includes unit prices. Otherwise it is not required and need not be included with the form. The number of unit prices that may be included is not limited and additional sheets may be included if needed.

** A CORPORATE RESOLUTION OR WRITTEN EVIDENCE of the authority of the person signing the bid for the public work as prescribed by LA R.S. 38:2212(B)(5).

BID SECURITY in the form of a bid bond, certified check or cashier's check as prescribed by LA R.S. 38:2218(A) attached to and made a part of this bid.

LOUISIANA UNIFORM PUBLIC WORK BID FORM UNIT PRICE FORM

<u>T0:</u>

St. Tammany Parish Government

21454 Koop Drive, Suite 2F

Mandeville, LA. 70471

(OWNER TO PROVIDE NAME AND ADDRESS OF OWNER)

Revised

BID FOR:

Bootlegger Road Improvements

Bid No. 300-00-18-66-2

Revised (owner to provide project name & other identifying info)

UNIT PRICES: This form shall be used for any & all work required by the Bidding Documents & described as unit prices. Amounts shall be stated in figures & only in figures.

Description:	☑ BASE BID OR	□ ALT #	REMOVAL OF BRIDGE (TIM	BER) (SUB/SUPERSTRUCTURE, & APPROACH SLAF
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
202-02-00100	1	LUMP		
Description:	☑ BASE BID OR	□ ALT #	REMOVAL OF CONCRETE V	/ALKS AND DRIVES
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
202-02-06100	120	SQYD		
Description:	☑ BASE BID OR	□ ALT #	REMOVAL OF CURBS (CON	CRETE)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
202-02-06140	120	LNFT		
Description:	☑ BASE BID OR	□ ALT #	REMOVAL OF GUARD RAIL	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
202-02-14500	287.00	LNFT		

Description	on: ☑ BASE BID OR	□ ALT #	REMOVAL OF SURFACING	AND STABILIZED BASE
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
202-02-38500	3,986.50	SQYD		
Description	on: ☑ BASE BID OR	□ ALT #	EXCAVATION AND EMBAN	KMENT
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
203-05-00100	1	LUMP		
Description	on: ☑ BASE BID OR	□ ALT #	TEMPORARY SEDIMENT CI	HECK DAMS
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
204-05-00100	74	EACH		
Description	on: ☑ BASE BID OR	□ ALT #	TEMPORARY SILT FENCINO	Ĵ
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
204-06-00100	8,200	LNFT		
Description	on: ☑ BASE BID OR	□ ALT #	CLASS II BASE COURSE	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
302-01-00100	4,035	CUYD		
Description	on: ☑ BASE BID OR	□ ALT #	LIME	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
304-01-00100 Version 2017 Q2	100	TON		

Descr	ption: ☑ BASE BID OR	□ ALT #	LIME TREATMENT (TYPE I	D) (12" THICK)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
304-04-00400	12,100	SQYD		
Descri	ption: 🗹 BASE BID 🛛 OR	□ ALT #	SUBGRADE LAYER (12")	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
305-01-04000	12,100	SQYD		
Descri	ption: ☑ BASE BID OR	\Box ALT #	NON-MAINLINE TRAFFIC M	AAINTENANCE SURFACING (AGGREGATE) (VEHICU
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
402-03-00100	200	CUYD		
Descri	ption: ☑ BASE BID OR	□ ALT #	ASPHALT CONCRETE	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
502-01-00100	7663.3	TON		
Descri	ption: 🗹 BASE BID 🛛 OR	□ ALT #	ASPHALT CONCRETE DRIV	ES, TURNOUTS AND MISCELLANEOUS
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
502-01-00200	16	TON		
Descri				
20001	ption: BASE BID OR		MILLING ASPHALI PAVEM	ENT (2" THICK)
REF NO.:	ption: ☑ BASE BID OR QUANTITY	UNIT OF MEASURE	UNIT PRICE	ENT (2" THICK) UNIT PRICE EXTENSION (Quantity times unit price)

Descriptio	n: 🗹 BASE BID 🛛 OR	\Box ALT #	PAVEMENT PATCHING (10	" MINIMUM THICKNESS)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
510-01-00104	300.00	SQYD		
Descriptio	n: ☑ BASE BID OR	□ ALT #	STORM DRAIN PIPE (15" R	PVCP)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
701-03-01002	33	LNFT		
Descriptio	n: ☑ BASE BID OR	\Box ALT #	STORM DRAIN PIPE (18" P	OLYMER COATED SPIRAL RIB CORRUGATED STEEL
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
701-03-01022	80	LNFT		
Descriptio	n: 🗹 BASE BID 🛛 OR	\Box ALT #	STORM DRAIN PIPE (36" P	OLYMER COATED SPIRAL RIB CORRUGATED STEEL
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
701-03-01082	650	LNFT		
Descriptio	n: ☑ BASE BID OR	□ ALT #	STORM DRAIN PIPE ARCH	(42" EQUIV. RCPA)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
701-04-01100	24	LNFT		
Descriptio	n: ☑ BASE BID OR	□ ALT #	STORM DRAIN PIPE ARCH	(48" EQUIV. RCPA)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)

Description	n: 🗹 BASE BID 🛛 OR	□ ALT #	STORM DRAIN PIPE (12" P	OLYMER COATED 2 2/3" x 1/2" CORRUGATED STEI
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
701-05-01001	70	LNFT		
Descriptio	n: 🗹 BASE BID 🛛 OR	□ ALT #	TRENCH EXCAVATION SAF	ETY PROTECTION (DEPTH >5 FEET)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
701-17-00100	665	LNFT		
Description	n: ☑ BASE BID OR	□ ALT #	MANHOLES (MH-06)	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
702-02-00100	5	EACH		
Description	n: 🗹 BASE BID 🛛 OR	\Box ALT #	CATCH BASIN (CB-01)	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
702-03-00100	1	EACH		
Descriptio	n: ☑ BASE BID OR	□ ALT #	CATCH BASIN (CB-02)	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
702-03-00200	6	EACH		
Descriptio	n: 🗹 BASE BID OR	□ ALT #	CATCH BASIN (CB-04)	-
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
702-03-00300 Version 2017 Q2	3	EACH		

Description	n: ☑ BASE BID OR	□ ALT #	BLOCKED OUT GUARDRAII	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
704-03-00100	450	LNFT		
Description	n: ☑ BASE BID OR	□ ALT #	GUARDRAIL TRANSITIONS	(DOUBLE THRIE BEAM)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
704-07-00200	100	LNFT		
Description	n: ☑ BASE BID OR	\Box ALT #	GUARDRAIL END TREATM	ENT (FLARED)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
704-10-00100	2	EACH		
Description	n: ☑ BASE BID OR	\Box ALT #	GUARDRAIL END TREATM	ENT (TANGENT)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
704-10-00200	2	EACH		
Description				
	n: 🖻 BASE BID OR		CONCRETE WALK (4" THIC	K)
REF NO.:	QUANTITY	UNIT OF MEASURE	CONCRETE WALK (4" THIC	K) UNIT PRICE EXTENSION (Quantity times unit price)
REF NO.: 706-01-00100	Image: Image Base BID OR QUANTITY 90	UNIT OF MEASURE SQYD	CONCRETE WALK (4" THIC	K) UNIT PRICE EXTENSION (Quantity times unit price)
REF NO.: 706-01-00100 Description	Image: Image: BASE BID OR QUANTITY 90 Image: Image: BASE BID OR	UNIT OF MEASURE SQYD	CONCRETE WALK (4" THIC UNIT PRICE CONCRETE DRIVE (6" THIC	K) UNIT PRICE EXTENSION (Quantity times unit price) CK)
REF NO.: 706-01-00100 Description REF NO.:	Image: Image Base Bid OR QUANTITY 90 Image Base Bid OR QUANTITY QUANTITY	UNIT OF MEASURE SQYD ALT # UNIT OF MEASURE UNIT OF MEASURE	CONCRETE WALK (4" THIC UNIT PRICE CONCRETE DRIVE (6" THIC UNIT PRICE	K) UNIT PRICE EXTENSION (Quantity times unit price) CK) UNIT PRICE EXTENSION (Quantity times unit price)

Description	: ☑ BASE BID OR	□ ALT #	RIPRAP (55 LB, 18" THICK)	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
711-01-04000	720	SQYD		
Description	: ☑ BASE BID OR	□ ALT #	GEOTEXTILE FABRIC	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
711-04-00100	720	SQYD		
Description	: ☑ BASE BID OR	□ ALT #	TEMPORARY SIGNS AND B	ARRICADES
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
713-01-00100	1	LUMP		
Description	: 🗹 BASE BID 🛛 OR	□ ALT #	MULCH (VEGETATIVE)	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
716-01-00100	1.4	TON		
Description	: ☑ BASE BID OR	□ ALT #	SEEDING	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
717-01-00100	21.6	LB		
Description	: ☑ BASE BID OR	□ ALT #	FERTILIZER	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
718-01-00100 Version 2017 02	720	LB		

Description	☑ BASE BID OR	□ ALT #	RUMBLE STRIPS (SHOULD	ER/EDGE)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
724-02-00100	6.286	MILE		
Description	■ BASE BID OR	□ ALT #	MOBILIZATION	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
727-01-00100	1	LUMP		
Description	BASE BID OR	□ ALT #	OBJECT MARKER ASSEMBI	Y (TYPE 3)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
729-16-00300	4	EACH		
Description	I 🗹 BASE BID OR	□ ALT #	SIGN (TYPE A)	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
729-01-00100	16.8	SQFT		
Description	BASE BID OR	□ ALT #	SQUARE TUBING POST WI	TH 2-1/4" ANCHOR
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
729-22-00100	2	EACH		
Description	BASE BID OR	□ ALT #	REFLECTORIZED RAISED P	AVEMENT MARKERS
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
731-02-00100	796	EACH		

Description	: ☑ BASE BID OR	□ ALT #	PLASTIC PAVEMENT STRIP	ING (SOLID LINE)(4" WIDTH)(THERMOPLASTIC 90
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-02-02000	10.197	MILE		
Description	: 🗹 BASE BID OR	□ ALT #	PLASTIC PAVEMENT STRIP	ING (SOLID LINE) (6" WIDTH) (THERMOPLASTIC 9
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-02-02020	4.349	MILE		
Description	: ☑ BASE BID OR	□ ALT #	PLASTIC PAVEMENT STRIP	ING (SOLID LINE) (8" WIDTH) (THERMOPLASTIC 9
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-02-02040	0.132	MILE		
Description	: ☑ BASE BID OR	□ ALT #	PLASTIC PAVEMENT STRIP	ING (SOLID LINE) (12" WIDTH) (THERMOPLASTIC
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-02-02060	0.411	MILE		
Description				
	BASE BID OR		PLASTIC PAVEMENT STRIP	ING (SOLID LINE) (24" WIDTH) (THERMOPLASTIC
REF NO.:	QUANTITY	UNIT OF MEASURE	PLASTIC PAVEMENT STRIP	ING (SOLID LINE) (24" WIDTH) (THERMOPLASTIC UNIT PRICE EXTENSION (Quantity times unit price)
REF NO.: 732-02-02080	QUANTITY 0.209	UNIT OF MEASURE MILE	PLASTIC PAVEMENT STRIP UNIT PRICE	ING (SOLID LINE) (24" WIDTH) (THERMOPLASTIC UNIT PRICE EXTENSION (Quantity times unit price)
REF NO.: 732-02-02080 Description	QUANTITY 0.209 : ☑ BASE BID OR	UNIT OF MEASURE MILE	PLASTIC PAVEMENT STRIP UNIT PRICE PLASTIC PAVEMENT STRIP	ING (SOLID LINE) (24" WIDTH) (THERMOPLASTIC UNIT PRICE EXTENSION (Quantity times unit price) ING (BROKEN LINE) (4" WIDTH) (THERMOPLASTI
REF NO.: 732-02-02080 Description REF NO.:	Image: Image	UNIT OF MEASURE MILE	PLASTIC PAVEMENT STRIP UNIT PRICE PLASTIC PAVEMENT STRIP UNIT PRICE	ING (SOLID LINE) (24" WIDTH) (THERMOPLASTIC UNIT PRICE EXTENSION (Quantity times unit price) ING (BROKEN LINE) (4" WIDTH) (THERMOPLASTI UNIT PRICE EXTENSION (Quantity times unit price)

Descriptio	n: ☑ BASE BID OR	□ ALT #	PLASTIC PAVEMENT STRIP	PING (DOTTED LINE)(8" W)(2' L)(THERMO 90 MIL)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-03-02030	0.108	MILE		
Descriptio	n: ☑ BASE BID OR	□ ALT #	PLASTIC PAVEMENT STRIP	PING (DOTTED LINE)(24" W)(2' L)(THERMO 90 MIL
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-03-02060	0.045	MILE		
Descriptio	n: 🗹 BASE BID 🛛 OR	□ ALT #	PLASTIC PAVEMENT LEGE	NDS AND SYMBOLS (ARROW - DOUBLE)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-04-01040	7	EACH		
Descriptio	n: ☑ BASE BID OR	□ ALT #	PLASTIC PAVEMENT LEGE	NDS AND SYMBOLS (ARROW - LEFT TURN)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-04-01080	3	EACH		
Descriptio	n: ☑ BASE BID OR	□ ALT #	PLASTIC PAVEMENT LEGE	NDS AND SYMBOLS (ARROW - RIGHT TURN)
REF NO.:	n: 🗹 BASE BID OR QUANTITY	ALT # UNIT OF MEASURE	PLASTIC PAVEMENT LEGE	NDS AND SYMBOLS (ARROW - RIGHT TURN) UNIT PRICE EXTENSION (Quantity times unit price)
REF NO.: 732-04-01100	n: BASE BID OR QUANTITY 4	□ ALT # UNIT OF MEASURE EACH	PLASTIC PAVEMENT LEGE	NDS AND SYMBOLS (ARROW - RIGHT TURN) UNIT PRICE EXTENSION (Quantity times unit price)
REF NO.: 732-04-01100 Descriptio	n: ☑ BASE BID OR QUANTITY 4 1: ☑ BASE BID OR	 ALT # UNIT OF MEASURE EACH ALT # 	PLASTIC PAVEMENT LEGE UNIT PRICE	NDS AND SYMBOLS (ARROW - RIGHT TURN) UNIT PRICE EXTENSION (Quantity times unit price) YMB (DIR ARR RNDBT - FSHK) (TYPE LTRC)
REF NO.: 732-04-01100 Descriptio REF NO.:	n: ☑ BASE BID OR QUANTITY 4 1: ☑ BASE BID OR QUANTITY	 ALT # UNIT OF MEASURE EACH ALT # UNIT OF MEASURE 	PLASTIC PAVEMENT LEGE UNIT PRICE PLSTC PVMT LGNDS AND S UNIT PRICE	NDS AND SYMBOLS (ARROW - RIGHT TURN) UNIT PRICE EXTENSION (Quantity times unit price) YMB (DIR ARR RNDBT - FSHK) (TYPE LTRC) UNIT PRICE EXTENSION (Quantity times unit price)

Description	BASE BID OR	□ ALT #	PLASTIC PAVEMENT LEGE	NDS AND SYMBOLS (ONLY)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-04-15020	7	EACH		
Description	BASE BID OR	□ ALT #	PLASTIC PAVEMENT LEGE	NDS AND SYMBOLS (YIELD)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-04-25000	5	EACH		
Description	I I BASE BID OR	□ ALT #	PLASTIC PAVEMENT LEGE	NDS AND SYMBOLS (YIELD LINE)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
732-04-25020	25	EACH		
Description	: ☑ BASE BID OR	□ ALT #	MAILBOXES	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
735-01-00100	17	EACH		
Description	I I BASE BID OR	□ ALT #	MAILBOX SUPPORTS (SING	LE)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
735-02-00100	15	EACH		
Description	: ☑ BASE BID OR	□ ALT #	MAILBOX SUPPORTS (DOU	BLE)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
735-03-00100 Version 2017 Q2	1	EACH		

Descriptio	n: ☑ BASE BID OR	□ ALT #	CONSTRUCTION LAYOUT	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
740-01-00100	1	LUMP		
Descriptio	n: ☑ BASE BID OR	□ ALT #	PRECAST CONCRETE PILES	5 (16")
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
804-01-00300	2,000	LNFT		
Descriptio	n: ☑ BASE BID OR	□ ALT #	DYNAMIC MONITORING AS	SISTANCE
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
804-14-00100	2	EACH		
Descriptio	n: 🗹 BASE BID 🛛 OR	\Box ALT #	VIBRATION MONITORING	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
804-18-00100	20	DAY		
Descriptio	n: ☑ BASE BID OR	□ ALT #	CLASS A1 CONCRETE (BEN	T CAP)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
805-01-00300	48	CUYD		
Descriptio	n: ☑ BASE BID OR	□ ALT #	CLASS A2 CONCRETE	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
805-02-00900 Version 2017 Q2	166	CUYD		

Description	:: ☑ BASE BID OR	□ ALT #	DEFORMED REINFORCING	STEEL
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
806-01-00100	67,443	LB		
Description	∷ 🗹 BASE BID OR	□ ALT #	CONCRETE BRIDGE RAILIN	IG (SLOTTED BARRIER)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
810-01-00200	94	LNFT		
Description	E BASE BID OR	□ ALT #	CONCRETE BRIDGE RAILIN	IG
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
810-01-00400	110	LNFT		
Description	∷ 🗹 BASE BID OR	□ ALT #	CONCRETE AND METAL BE	RIDGE RAILING
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
810-03-00400	290	LNFT		
Description	E BASE BID OR	□ ALT #	CONCRETE APPROACH SLA	ABS
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
813-01-00100	206	SQYD		
Description	E BASE BID OR	□ ALT #	SEAL EXPANSION JOINT	-
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
815-02-00500 Version 2017 Q2	98	LNFT		

Description	∷ 🗹 BASE BID OR	□ ALT #	GEOGRID	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
NS-300-00060	13,030	SQYD		
Description	∷ 🗹 BASE BID OR	□ ALT #	POLYMER BASED ASPHAL	Γ REINFORCEMENT INTERLAYER
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
S-002	54,836	SQYD		
Description	∷ 🗹 BASE BID OR	\Box ALT #	POLYMER COATED CMP SA	ADDLE TEE (36" X 12")
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
S-003	3	EACH		
Description	II: ☑ BASE BID OR	\Box ALT #	ASPHALT COATED CMP EN	ID SECTION (12")
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
S-004	7	EACH		
Description	I: ☑ BASE BID OR	\Box ALT #	TRASH GUARDS FOR CM E	ND SECTIONS
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
S-005	7	EACH		
Description	⊫ □ BASE BID OR	☑ ALT #	ASPHALT CONCRETE	
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
502-01-00100 Version 2017 Q2	4,700	TON		

Description	BASE BID OR	☑ ALT #	MILLING ASPHALT PAVEM	ENT (2" THICK)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
509-01-00100	-50,850	SQYD		
Description	BASE BID OR	☑ ALT #	MILLING ASPHALT PAVEM	ENT (3.5" THICK)
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
509-01-00200	50,850	SQYD		
Description	BASE BID OR	☑ ALT #	POLYMER BASED ASPHALT	F REINFORCEMENT INTERLAYER
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
S-002	-50,000	SQYD		
Description	BASE BID OR	□ ALT #		
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
Description	BASE BID OR	□ ALT #		
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
Description	BASE BID OR	□ ALT #		
REF NO.:	QUANTITY	UNIT OF MEASURE	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times unit price)
Version 2017 Q2				





ST. TAMMANY PARISH PATRICIA P. BRISTER PARISH PRESIDENT

January 24, 2019

Please find the following addendum to the below mentioned BID.

Addendum No.: 1

Bid#: 300-00-18-66-2

Project Name: Bootlegger Road Improvements

Bid Due Date: Thursday, February 14, 2019

GENERAL INFORMATION:

- Please note that the Bid Opening has been pushed to Thursday, February 14, 2019. Time and location remain the same. The last Day to submit inquiries is Tuesday, February 5, 2019 at 2:00pm. The last day for an Addendum is Monday, February 11, 2019 is at 2:00pm.
- 2. Mandatory Pre-bid Meeting Sign in sheet. (Attached)

ATTACHMENTS:

1. Mandatory Pre-Bid Sign in Sheet.pdf

End of Addendum #1

PROCUREMENT DEPARTMENT P.O. BOX 628 | COVINGTON, LOUISIANA | 70434 | PROCUREMENT@STPGOV.ORG | 985-898-2520 WWW.STPGOV.ORG

Location: Staff Conference Rm. 3rd Floor

21454 (Building B) Koop Dr., Mandeville, LA 70471

Pre-Bid Sign-In Sheet Bootlegger Road Improvements

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	Korta Construction Service	Mage Wcawation	Barriere Construction Cay LLC	Industrial Fabrics	Diamond B	JB James Construction LLC	QSM	Cleco	Guinn Construction	Beverly Const.	1 Er san	m Davald Corst.	Company
SERVICES .DOM	SFOXWORTHE KORTSCONSTRUCTON	Stip Omagelycavation of	jacobm@barriere,com	tpena ind-fab.com	Cecil. Huches@pigmonBlue	ross C @ byames Il c, com	Cric Qquelitus tewark nate	stacey, deshota @ checo.com	MHAKE GUINN CONST AUCT 2004 LLC. Com	CANNY@ Beverlyinc.com	M return a war of a	moncus. me donald construction & yohus com	Email
	(985) 590-0985	rt 985-892-W	504-421-0819	225 - 9077116	B67-1/14526	225-927-3181	504-908-8401	3.37-501-2688	337-824-0632	985-264-8725	985-501-0273	985-641-3621	Phone
	1:55	01 1:50	1:50	1:42	1:51	1: 50	1:45	1751	1:45	-1	1:43	1:43	Time In
	2:23	2,23	J:20	2:20	2:21	2:20	1:10	2:23	2:24	2:21	2:20	2.21	Time Out

	13	14	15	16	17	18	19	20	21	22	23	24	25	26
Name	KYLE HARTZOG	Jason Cambre	Anthony MATHERE	Unxul Hardy	Tim other MANN									
Company	KURTS Construction Services	St Tammany Parish bout	CLECO	St. Tammany Pation	Stranco, L.L.C.									
Email	Knew trang @ Kosts construction	jp combine @stpgov.org	ANTHONY, MATHERNE @ OLECO. don	Lienardy@stppov.org	timper @ Stranco.net									
Phone	(985-515-1842	022-858-2525	985-807-6727		985-630-4667									
Time In	1:55	١،	2:00	2:00	1:50									
Time Out	2:23	56:6	2:23	1:24	2:18									